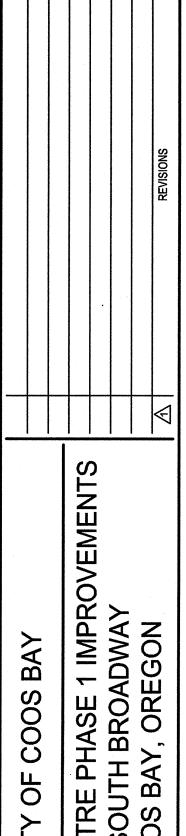
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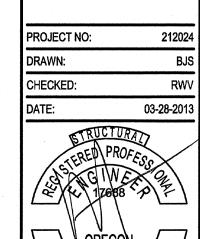
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ABBREVIATIONS

A.B.	ANCHOR BOLT	FIN.	FINISH	PART.	PARTITION
ACI	AMERICAN CONCRETE INSTITUTE	FLR.	FLOOR	P/C	PRECAST
ADD'L.	ADDITIONAL	FT.	FOOT	PCF	POUNDS PER CUBIC FOOT
AESS	ARCHITECTURAL EXPOSED	FTG.	FOOTING	PL	PLATE
ALCO	STRUCTURAL STEEL	GA.	GAUGE	P.P.	PARTIAL PENETRATION
AISC	AMERICAN INSTITUTE OF	GALV.	GALVANIZED	PSI	POUNDS PER SQUARE INCH
	STEEL CONSTRUCTION INCORPORATED	GL	GLULAM	P/T	POST-TENSIONED
ALT.	ALTERNATE	HORIZ.	HORIZONTAL	P.T.	PRESSURE TREATED
ALUM.	ALUMINUM	HSS	HOLLOW STRUCTURAL SECTION	PVC	POLYVINYL CHLORIDE
APA	AMERICAN PLYWOOD ASSOCIATION	IBC	INTERNATIONAL BUILDING CODE	R, RAD. RCSC	RADIUS RESEARCH COUNCIL ON
ARCH.	ARCHITECT	ICBO	INTERNATIONAL CONFERENCE		STRUCTURAL CONNECTIONS
ASCE	AMERICAN SOCIETY OF CIVIL		OF BUILDING OFFICIALS	REF.	REFERENCE
AOTM	ENGINEERS	ICC	INTERNATIONAL CODE	RET.	RETURN
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	I.D.	COUNCIL INSIDE DIAMETER	REINF.	REINFORCING
AWS	AMERICAN WELDING SOCIETY	IN.	INCH	REQ'D.	REQUIRED
BLDG.	BUILDING	INT.	INTERIOR	REQ'MTS.	REQUIREMENTS
вот.	ВОТТОМ	K	KIPS	SCHED.	SCHEDULE
BRBF	BUCKLING RESTRAINED	KSF	KIPS PER SQUARE FOOT	S.C.	SLIP CRITICAL
	BRACED FRAME	KSI	KIPS PER SQUARE INCH	SIM.	SIMILAR
C.G.	CENTER OF GRAVITY	LBS.	POUND	SLRS	SEISMIC LOAD RESISTING SYSTEM
C.I.P.	CAST IN PLACE	L.L.	LIVE LOAD	S.O.G.	SLAB ON GRADE
C.J.	CONTROL JOINT	LLH	LONG LEG HORIZONTAL	SPEC.	SPECIFICATION
C.J.P.	COMPLETE JOINT PENETRATION	LLV	LONG LEG VERTICAL	SQ.	SQUARE
CL	CENTERLINE	LOC.	LOCATION	SS	STAINLESS STEEL
CLR.	CLEAR	LONG.	LONGITUDINAL	SSMA	STEEL STUD
СМИ	CONCRETE MASONRY UNIT	LVF	LOW VELOCITY FASTENER		MANUFACTURERS
COL.	COLUMN	MAX.	MAXIMUM	STD.	ASSOCIATION STANDARD
CONC.	CONCRETE	MBMA	METAL BUILDING	STRUCT.	STRUCTURAL
CONN.	CONNECTION		MANUFACTURERS	SYM.	SYMMETRICAL
CONST.	CONSTRUCTION	MECH	ASSOCIATION	THRU	THROUGH
CONT.	CONTINUOUS	MECH.	MECHANICAL	T&G	TONGUE AND GROOVE
db	BAR DIAMETER	MFR.	MANUFACTURER	TJ	
DBA	DEFORMED BAR ANCHOR	MIN.	MINIMUM	TRANS.	TRUSS JOIST TRANSVERSE
DET.	DETAIL	MISC.	MISCELLANEOUS	TRANS.	LIGHT GAUGE TUBE STEEL
DIA., Ø	DIAMETER	MPH	MILES PER HOUR	TYP.	TYPICAL
DIAG.	DIAGONAL	MT	MAGNETIC PARTICLE TESTING	U.N.O.	UNLESS NOTED OTHERWISE
D.L.	DEAD LOAD	(N)	NEW	UT	ULTRASONIC TESTING
DWG.	DRAWING	N.I.C.	NOT IN CONTRACT	VERT.	VERTICAL
ELEC.	ELECTRICAL	NOM.	NOMINAL	VERT. V.I.F.	VERIFY IN FIELD
EL.	ELEVATION	NO.	NUMBER	w/	WITH
EQ.	EQUAL	N.T.S.	NOT TO SCALE	w/ WF	
EXIST.,	EXISTING	0.C.	ON CENTER		WIDE FLANGE
(E)		O.D.	OUTSIDE DIAMETER	w/o	WITHOUT
EXP.	EXPANSION	OPP.	OPPOSITE	W.P.	WORK POINT
EXT.	EXTERIOR	OWJ	OPEN WEB JOIST	WPS	WELDING PROCEDURE SPECIFICATION
FDN.	FOUNDATION	PAF	POWDER ACTUATED FASTENER	WWF	WELDED WIRE FABRIC







100% CONSTRUCTION
DOCUMENTS

DRAWING INDEX
AND LIST OF
ABBREVIATIONS

0.1

THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.

CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.

ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS SHALL BE FIELD VERIFIED. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY SIGNIFICANT DISCREPANCIES FROM CONDITIONS SHOWN ON THE DRAWINGS.

ASSUMED FUTURE CONSTRUCTION:

VERTICAL: NONE HORIZONTAL: NONE

CODE REQUIREMENTS:

THE SCOPE OF STRUCTURAL WORK IN THESE DRAWINGS IS LIMITED TO THE UPGRADE OF GRAVITY SUPPORT ELEMENTS THAT EXCEED THE CRITERIA OF CITY OF COOS BAY MUNICIPAL CODE SECTION 15.15, "DANGEROUS BUILDINGS". ALL NEW GRAVITY ELEMENTS IN THESE DRAWINGS CONFORM TO THE 2010 OREGON STRUCTURAL SPECIALTY CODE (OSSC), BASED ON THE 2009 INTERNATIONAL BUILDING CODE (IBC).

GRAVITY ELEMENT IMPROVEMENTS DO NOT REQUIRE A SEISMIC UPGRADE UNDER THE PROVISIONS OF THE OSSC. ALL SEISMIC IMPROVEMENTS ARE VOLUNTARY. AND WERE DESIGNED TO CONFORM TO THE BASIC SAFETY OBJECTIVE OF ASCE 41-06 "SEISMIC REHABILITATION OF EXISTING BUILDINGS". THE SCOPE OF THE VOLUNTARY SEISMIC WORK WAS LIMITED TO THE ADDITION OF STEEL STRONGBACKS AT THE WEST WALL AND DIAPHRAGM STRENGTHENING AT THE HIGH ROOF (FLY-LOFT). REFER TO DESIGN CRITERIA OR ADDITIONAL INFORMATION.

DESIGN CRITERIA:

DESIGN WAS BASED ON THE STRENGTH AND DEFLECTION CRITERIA OF THE OSSC, IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LOADS AND ALLOWABLES WERE USED FOR DESIGN, WITH LIVE LOADS (L.L.) REDUCED

	DECICN CDITEDIA	·	
	DESIGN CRITERIA		
GRA\	/ITY SYSTEM CRITI	ERIA	
ROOF LIVE/SNOW LOAD	25 PSF L.L. (ALSO SEE SNOW LO	DAD CRITERIA BELOW)	
FLOOR LIVE LOADS:	UNIFORM LOAD	CONCENTRATED LOAD	
CORRIDORS AND STAIRS	100 PSF L.L.	2,000 LBS.	
ASSEMBLY AREAS, RETAIL	100 PSF L.L.	2,000 LBS.	
VERTICAL FLOOR DEFLECTION (INTERIOR)	L/360 LIVE LOAD PER OSSO	C TABLE 1604.3	
NOTES:	LIVE LOADS REDUCED PER OSSC.		
	2. MEMBER DESIGNED FOR MORE CRITICA CONCENTRATED LOAD.	L OF UNIFORM OR	
	SNOW CRITERIA		
DESIGN ROOF SNOW LOAD	25 PSF MINIMUM IN ACCORDA	ANCE WITH OSSC	
SNOW DRIFT	PER OSSC AS SHOWN	ON PLANS	
GROUND SNOW LOAD	Pg= 10 PSF IN ACCORDANCE WITH 2007 SNOW LOAD ANALYSIS FOR OREGON		
FLAT ROOF SNOW LOAD	Pf = 14 PSF		
SNOW EXPOSURE FACTOR	Ce = 1.0		
SNOW LOAD IMPORTANCE FACTOR	I = 1.1		
THERMAL FACTOR	Ct = 1.0		
GEO	TECHNICAL CRITE	RIA	
DESIGN BASED ON REPORT BY:	SHN, INC. DATED DECEM	IBER 3, 2012	
	WIND CRITERIA		
MAIN WIND FORCE RESISTING SYSTEM	100 MPH BASIC WIND SPEED (3-SECOND GUST)		
COMPONENTS AND CLADDINGS	100 MPH BASIC WIND SPEED (3-SECOND GUST)		
EXPOSURE CATEGORY	С		
IMPORTANCE FACTOR	lw = 1.15		
GUST/INTERNAL PRESSURE	GCpi = +/- 0.18		
	SEISMIC CRITERIA		
OCCUPANCY CATEGORY	III		
SITE CLASS	E CLASS E (ASSUMED)		

BASIC SAFETY EARTHQUAKE - 1 (10% IN 50 YEARS)

SPECTRAL ACCELERATION PARAMETER	Ss = 0.6 g
SITE COEFFICIENT	Fa = 1.5
DESIGN SPECTRAL ACCELERATION	Sxs = 0.9 g

BASIC SAFETY EARTHQUAKE - 2 (2% IN 50 YEARS)

SPECTRAL ACCELERATION PARAMETER	Ss = 1.50 g
SITE COEFFICIENT	Fa = 0.9
DESIGN SPECTRAL ACCELERATION	Sxs = 1.50 g

THE STRUCTURAL ENGINEER OF RECORD (SER) WILL PERFORM STRUCTURAL OBSERVATION BASED ON THE REQUIREMENTS OF THE OSSC AT THE STAGES OF CONSTRUCTION LISTED BELOW. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SER TO PERFORM THESE OBSERVATIONS.

STRUCTURAL OBSERVATIONS			
ITEM	OBSERVED BY (2)	COMMENTS	
ITEM	SER	COMMENTS	
DURING DEMOLITION	Х	REF. NOTE 6	
PRIOR TO FIRST CONCRETE POUR	Х	REF. NOTES 1,3,4,5	
DURING INITIAL STEEL ERECTION	Х	REF. NOTES 1,3,4	
AS REQUIRED TO ADDRESS STRUCTURAL ISSUES	X	REF. NOTES 1,3,4	

FOOTNOTES:

CONTRACTOR IS RESPONSIBLE FOR NOTIFYING THE SER IN ADVANCE.

- SER STRUCTURAL ENGINEER OF RECORD.
- 3. A FIELD REPORT WILL BE SUBMITTED TO THE BUILDING DEPARTMENT FOLLOWING EACH SITE VISIT.
- 4. STRUCTURAL OBSERVATION IS FOR THE GENERAL CONFORMANCE OF THE STRUCTURAL DRAWING, SPECIAL INSPECTION IS STILL REQUIRED.
- 5. AFTER REINFORCING STEEL HAS BEEN INSTALLED.
- 6. CONTRACTOR SHALL COORDINATE A SITE VISIT WITH STRUCTURAL ENGINEER OF RECORD TO REVIEW AS BUILT CONDITIONS AND CONFIRM MEMBER SIZES AND CONFIGURATION FOR EXISTING ELEMENTS PRIOR TO PROCEEDING WITH ANY NEW WORK.

SPECIAL INSPECTION WILL BE PROVIDED BY THE OWNER BASED ON THE REQUIREMENTS OF THE OSSC AS SUMMARIZED IN THE SPECIAL INSPECTION AND TESTING PROGRAM ON SHEET S0.3. CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE AND ACCESS FOR THE SPECIAL INSPECTOR TO PERFORM THESE INSPECTIONS.

SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO FABRICATION AND CONSTRUCTION OF ALL STRUCTURAL ITEMS, INCLUDING THE FOLLOWING:

SUBMITTALS				
ITEM	SUBMITTAL (1,4)	DEFERRED SUBMITTAL (2,4)	COMMENTS	
PIN PILES/MICRO PILES	Х	Х		
CONCRETE MIX DESIGNS	Х			
CONCRETE REINFORCEMENT	Х			
CONCRETE ANCHORAGES	X		-	
EMBEDDED STEEL ITEMS	х			
STRUCTURAL STEEL	Х			
STEEL WELDING PROCEDURES	Х			

FOOTNOTES:

- SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO FABRICATION AND CONSTRUCTION OF STRUCTURAL ITEMS. IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON. ANY CHANGES TO THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER AND ARE SUBJECT TO REVIEW AND ACCEPTANCE OF THE STRUCTURAL ENGINEER.
- 2. DESIGN DRAWINGS, SHOP DRAWINGS, AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON, AND SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO FABRICATION. CALCULATIONS SHALL BE INCLUDED FOR ALL CONNECTIONS TO THE STRUCTURE, CONSIDERING LOCALIZED EFFECTS ON STRUCTURAL ELEMENTS INDUCED BY THE CONNECTION LOADS. DESIGN SHALL BE BASED ON THE REQUIREMENTS OF THE OSSC AND AS NOTED UNDER "DESIGN CRITERIA".
- THE CONTRACTOR SHALL COORDINATE SEISMIC RESTRAINTS OF MECHANICAL, PLUMBING, AND ELECTRICAL EQUIPMENT, MACHINERY, AND ASSOCIATED PIPING WITH THE STRUCTURE. CONNECTIONS TO STRUCTURE SHALL CONFORM TO ASCE 7-05 CHAPTER 13, BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF OREGON, AND SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO FABRICATION.
- 4. FIELD ENGINEERED DETAILS DEVELOPED BY THE CONTRACTOR THAT DIFFER FROM OR ADD TO THE STRUCTURAL DRAWINGS SHALL BEAR THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF OREGON AND SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO CONSTRUCTION.

MICRO PILES SHALL BE 12"Ø AND CAPABLE OF DEVELOPING A MINIMUM WORKING DESIGN LOAD OF 25 TONS DOWNWARD AND 5 TONS UPWARD. ANTICIPATED PILE LENGTH TO ACHIEVE THIS CAPACITY IS 100'-0". MICRO PILE CAPACITY SHALL BE VERIFIED BY TESTING AS REQUIRED BY THE SPECIFICATIONS.

CONCRETE WORK SHALL CONFORM TO CHAPTER 19 OF THE OSSC. CONCRETE STRENGTHS SHALL BE VERIFIED BY STANDARD 28-DAY CYLINDER TESTS PER ASTM C39, AND SHALL BE AS FOLLOWS:

CONCRETE STRENGTHS				
fc (PSI)	ABSOLUTE WATER-CEMENT RATIO BY WEIGHT		USE	
	NON AIR-ENTRAINED	AIR-ENTRAINED		
4,000	.50	.45	ALL USES	

VERIFY WATER/CEMENT RATIO WITH FLOOR COVERING MANUFACTURER FOR CONCRETE FLOORS WITH MOISTURE SENSITIVE FLOOR COVERINGS.

MINIMUM CEMENT CONTENT PER CURIC YARD SHALL BE AS FOLLOWS:

CEMENT CONTENT				
fc (PSI) MINIMUM CEMENT PER CUBIC YARD				
4,000	550 LBS.			

FLYASH CONFORMING TO ASTM C618 (INCLUDING TABLE 2A) TYPE F OR TYPE C MAY BE USED TO REPLACE UP TO 20% OF THE CEMENT CONTENT, PROVIDED THAT THE MIX STRENGTH IS SUBSTANTIATED BY TEST DATA.

THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS ALONG WITH TEST DATA COMPLIANT WITH OSSC SECTION 1905 A MINIMUM OF TWO WEEKS PRIOR TO PLACING CONCRETE. NO WATER MAY BE ADDED TO CONCRETE IN THE FIELD UNLESS SPECIFICALLY APPROVED IN WRITING BY THE CONCRETE SUPPLIER IN CONJUNCTION WITH THE CONCRETE MIX DESIGN.

A WATER-REDUCING ADMIXTURE CONFORMING TO ASTM C494 USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS SHALL BE INCORPORATED IN CONCRETE DESIGN MIXES. A HIGH-RANGE WATER-REDUCING (HRWR) ADMIXTURE CONFORMING TO ASTM C494 TYPE F OR G MAY BE USED IN CONCRETE MIXES PROVIDING THAT THE SLUMP DOES NOT EXCEED 10". AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260 SHALL BE USED IN CONCRETE MIXES FOR EXTERIOR HORIZONTAL SURFACES EXPOSED TO WEATHER. THE AMOUNT OF ENTRAINED AIR SHALL BE 6% ± 1% BY VOLUME.

SLEEVES, OPENINGS, CONDUIT, AND OTHER EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER BEFORE PLACING CONCRETE. CONDUITS EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIMENSION THAN ONE THIRD OF THE THICKNESS OF THE SLAB AND SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS ON CENTER.

WHERE NEW CONCRETE IS PLACED AGAINST EXISTING CONCRETE, THE EXISTING CONCRETE SURFACE SHALL BE CLEANED AND ROUGHENED TO A MINIMUM 1/4" AMPLITUDE. PROVIDE 3/4" CHAMFERS ON ALL EXPOSED CONCRETE EDGES, UNLESS NOTED OTHERWISE.

REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, FOR DEFORMED BARS AND ASTM A185 FOR SMOOTH WELDED WIRE FABRIC (WWF), UNLESS OTHERWISE NOTED. REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706. COLUMN SPIRALS SHALL BE PLAIN OR DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60. REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE WITH #16 ANNEALED IRON

BARS IN BEAMS AND SLABS SHALL BE SUPPORTED ON WELL-CURED CONCRETE BLOCKS OR APPROVED METAL CHAIRS, AS SPECIFIED BY THE CRSI MANUAL OF STANDARD PRACTICE, MSP-1. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE "ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," ACI 315. SHOP DRAWINGS SHALL INCLUDE ELEVATIONS OF ALL BEAMS, WALLS AND COLUMNS SHOWING BAR LOCATIONS. LAP ALL REINFORCING BARS PER THE TYPICAL LAP SPLICE LENGTH SCHEDULES, EXCEPT AS NOTED ON DRAWINGS. USE LAP LENGTH FOR SMALLER BAR WHEN SPLICING DIFFERENT BAR SIZES. MECHANICAL SPLICES NOTED ON THE PLANS SHALL BE DAYTON SUPERIOR BAR-LOCK (ICC ESR-2495) OR TAPERLOCK COUPLERS (ICC ESR-2481) OR APPROVED WITH A CURRENT ICC APPROVAL REPORT.

TYP. LAP SPLICE LENGTH SCHEDULE (IN.)				
(DOES NOT APPLY TO VERTICAL SHEAR WALL BOUNDARY REINFORCEMENT)				
BAR SIZE	0 PSI			
DAR SIZE	CASE 1	CASE 2		
#4	20	18		
#5	28	24		
#6	37	28		
#7	60	40		
#8	74	46		

- CASE 1 APPLIES TO BAR WITH CLEAR COVER < 1 1/2". CASE 2 APPLIES TO BAR WITH CLEAR COVER > 1 1/2".
- 2. FOR CENTER-TO-CENTER SPACING LESS THAN 4db MULTIPLY LAP LENGTHS ABOVE BY 1.3.
- 3. FOR TOP BARS, CAST ABOVE 12" OF CONCRETE, MULTIPLY LAP LENGTHS ABOVE BY 1.3.

REINFORCING STEEL CONCRETE COVER			
USE	COVER		
FOOTING BARS AND GRADE BEAMS	3"		

HEADED SHEAR STUDS SHALL BE NELSON HEADED ANCHORS WITH FLUXED ENDS (ICC ESR-2856) OR APPROVED. DEFORMED BAR ANCHORS (DBA) SHALL BE NELSON, TYPE D2L (ICC ESR-2907), OR APPROVED. STUDS AND DBA SHALL BE AUTOMATICALLY END-WELDED WITH THE MANUFACTURER'S STANDARD EQUIPMENT IN ACCORDANCE WITH THEIR RECOMMENDATIONS.

APPROVED POST INSTALLED ANCHORS				
ANCHORS	TYPE	ALTERNATE		
EXPANSION	HILTI KWIK BOLT TZ (ICC ESR-1917)	SIMPSON STRONG-BOLT 2 (ICC ESR-3037)		
CONCRETE SCREW	HILTI KWIK HUS-EZ (ICC ESR-3027)	SIMPSON TITEN HD (ICC ESR-2713)		
EPOXY ADHESIVE	HILTI HIT-RE 500SD (ICC ESR-2322)	SIMPSON SET-XP (ICC ESR-2508)		

ALL ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT REINFORCING IN NEW OR EXISTING CONCRETE DURING INSTALLATION. ANCHORS EXPOSED TO EARTH OR WEATHER SHALL BE PROTECTED FROM CORROSION BY HOT-DIP GALVANIZING OR USE OF STAINLESS

PERMANENTLY EXPOSED EMBEDDED PLATES AND ANGLES SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION, UNLESS OTHERWISE NOTED. NO LOADS OR WELDS SHALL BE PLACED ON EMBEDDED PLATES OR ANGLES FOR A MINIMUM OF 7 DAYS AFTER CASTING.

STRUC	CTURAL STEEL
ASTM A572, GRADE 50	PLATES WHERE NOTED
ASTM A36	CHANNELS, PLATES AND ANGLES, EXCEPT AS NOTED
ASTM A500, GRADE B (FY=46KSI)	HOLLOW STRUCTURAL SECTIONS (TUBES)

DESIGN, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" WITH "COMMENTARY" AND THE "CODE OF STANDARD PRACTICE", WITH EXCEPTIONS NOTED IN SPECIFICATIONS.

BOLTS SHALL CONFORM TO THE ASTM AND RCSC SPECIFICATIONS FOR JOINTS USING A325 OR A490 HIGH STRENGTH BOLTS. BOLTS SHALL BE SNUG-TIGHT UNLESS NOTED OTHERWISE.

WELDING SHALL CONFORM TO THE AWS CODES FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDED PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS D1.1 AND APPROVED BY THE STRUCTURAL ENGINEER. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER-METAL MANUFACTURER.

WELDS SHALL BE MADE USING E70XX ELECTRODES AND SHALL BE 3/16" MINIMUM, UNLESS OTHERWISE NOTED.

PROVIDE WEEP HOLES AT EXTERIOR CLOSED SECTIONS WHERE MOISTURE MAY ACCUMULATE. HOT DIP GALVANIZE ALL EXTERIOR STRUCTURAL STEEL.

SAWN LUMBER SHALL CONFORM TO WEST COAST LUMBER INSPECTION BUREAU OR WESTERN WOOD PRODUCTS ASSOCIATION GRADING RULES. LUMBER SHALL BE KILN DRIED AND BE THE SPECIES AND GRADE NOTED BELOW:

	SAWN LUMBER	
USE	SPECIES/GRADE	Fb (PSI) (BASE VALUE)
DIM. LUMBER 2" TO 4" THICK	DOUGLAS FIR-LARCH NO. 2	900
BEAMS 5"x5" AND GREATER	DOUGLAS FIR-LARCH NO. 1	1350
POSTS	DOUGLAS FIR-LARCH NO. 1	1200

ALL LUMBER IN CONTACT WITH CONCRETE OR CMU SHALL BE PRESSURE TREATED, UNLESS AN APPROVED MOISTURE BARRIER IS PROVIDED. FRAMING ACCESSORIES AND STRUCTURAL FASTENERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE (OR APPROVED EQUAL) AND OF THE SIZE AND TYPE SHOWN ON THE DRAWINGS. ALL NAIL HOLES SHALL BE FILLED WITH STRUCTURAL FASTENERS, UNLESS NOTED OTHERWISE ON THE DRAWINGS AND FASTENERS SHALL BE INSTALLED FOLLOWING ALL MANUFACTURERS REQUIREMENTS. IF A SUBSTITUTION IS MADE, A DOCUMENT SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL OUTLINING THE FRAMING ACCESSORIES BEING REPLACED AND THE SUBSTITUTED FRAMING ACCESSORIES. ALLOWABLE LOADS FOR THE SIMPSON ACCESSORIES SHALL BE TABULATED ALONG WITH ALLOWABLE LOADS FOR THE SUBSTITUTED ACCESSORIES, WHICH CLEARLY INDICATE THE SUBSTITUTED ACCESSORIES HAVING AN EQUAL OR GREATER CAPACITY. HANGERS NOT SHOWN SHALL BE SIMPSON U-TYPE OR B-TYPE OF SIZE RECOMMENDED FOR THE MEMBER.

ALL FRAMING NAILS SHALL BE OF THE SIZE AND NUMBER INDICATED ON THE DRAWINGS AND CONFORM TO ASTM F 1667, "STANDARD SPECIFICATION OF DRIVEN FASTENERS: NAILS, SPIKES, AND STAPLES AND NER-272 "POWER-DRIVEN STAPLES AND NAILS FOR USE IN ALL TYPES OF BUILDING CONSTRUCTION." NAILS SHALL BE IDENTIFIED BY LABELS (ATTACHED TO THEIR CONTAINERS) THAT SHOW THE MANUFACTURER'S NAME AND NES REPORT NUMBER, NAIL SHANK DIAMETER, AND LENGTH AND SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO FRAMING. NAILING NOT SHOWN SHALL BE AS INDICATED ON OSSC TABLE 2304.9.1 OR NER-272. THE FOLLOWING NAIL SIZES SHALL BE USED:

FRAMING NAILS								
NAIL TYPE	SHANK DIAMETER (IN.)	MINIMUM PENETRATION INTO FRAMING MEMBER (IN.)						
6d	0.113	1.250						
8d	0.131	1.500						
10d	0.148	1.625						
12d	0.148	1.625						
16d	0.148	1.625						

BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981. ALL BOLTS AND LAG SCREWS SHALL BE INSTALLED WITH STANDARD CUT WASHERS. ALL A307 BOLTS SHALL HAVE CUT THREADS.

CUTTING AND NOTCHING OF JOISTS AND STUDS SHALL CONFORM TO OSSC SECTIONS 2308.8.2, 2308.9.10 AND

SALVAGED LUMBER SHALL BE GRADED BY AN APPROVED GRADING AGENCY PRIOR TO USE AND SHALL MEET THE MINIMUM BENDING STRESSES SHOWN ABOVE.

WOOD STRUCTURAL PANELS SHALL CONFORM TO THE REQUIREMENTS OF "U.S. PRODUCT STANDARD PS 1 FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD", "U.S. PRODUCT STANDARD PS 2 PERFORMANCE STANDARD FOR WOOD-BASED STRUCTURAL-USE PANELS", OR "APA PRP-108 PERFORMANCE STANDARDS". UNLESS NOTED. PANELS SHALL BE APA STRUCTURAL 1 RATED SHEATHING, EXPOSURE 1, OF THE THICKNESS AND SPAN RATING SHOWN ON THE DRAWINGS.

WOOD STRUCTURAL PANEL INSTALLATION SHALL BE IN CONFORMANCE WITH APA RECOMMENDATIONS. ALLOW 1/8" SPACING AT PANEL ENDS AND EDGES, UNLESS OTHERWISE RECOMMENDED BY THE PANEL

ALL ROOF SHEATHING SHALL BE INSTALLED WITH FACE GRAIN PERPENDICULAR TO SUPPORTS, EXCEPT AS INDICATED ON THE DRAWINGS. ROOF SHEATHING SHALL EITHER BE BLOCKED, OR HAVE EDGES CENTERED ON EXISTING STRAIGHT SHEATHING BOARDS. WHEN ROOF SHEATHING IS NAILED DIRECTLY TO BLOCKING. THE BLOCKING SHALL BE NAILED TO SUPPORT MEMBERS WITH A MINIMUM OF 16d NAILS AT 4" O.C. ALL NAILS SHALL BE COMMON NAILS EXCEPT USE RING SHANK FOR ROOF SHEATHING.

PROJECT NO:

100% CONSTRUCTION **DOCUMENTS**

S0.2

STRUCTURAL

SPECIAL INSPECTIONS AND TESTING PROGRAM

TABLE 1 - REQUII	RED GE	OTECHNIC	AL SPE	ECIAL	INSPECTIONS
		INSPECTIO			
SYSTEM OR MATERIAL	IBC CODE REFERENCE	CODE OR STANDARDS	FREQUENCY	Y (NOTE 5)	REMARKS
		REFERENCE	CONTINUOUS	PERIODIC	
		SOILS			
VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY				×	
VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	1704.7	GEOTECHNICAL REPORT		Х	BY THE GEOTECHNICAL ENGINEER
PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS		, na on		×	
VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL			х		
		MICRO PILE PLACEMEN	NT .		
OBSERVE OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH MICRO PILE			×		
VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM DIAMETERS, REINFORCING, LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE). RECORD CONCRETE OR GROUT VOLUMES	1704.9	GEOTECHNICAL REPORT	х		BY THE GEOTECHNICAL ENGINEER
FOR CONCRETE MICRO PILES, PERFORM ADDITIONAL INSPECTIONS	1704.4				REFER TO TABLE 2 - CONCRETE FOR ADDITIONAL MATERIALS RELATED SPECIAL INSPECTIONS AND TABLE 3 FOR TESTING BY SPECIAL INSPECTOR

TABLE 2 - REQU	IRED S	TRUCTURA	AL SPEC	CIAL II	NSPECTIONS
	INSPECTION				
SYSTEM OR MATERIAL	IBC CODE REFERENCE	CODE OR STANDARD	FREQUENCY (NOTE 5)		REMARKS
	<u> </u>	REFERENCE	CONTINUOUS	PERIODIC	
		FABRICATORS			
FABRICATORS	1704.2		x		WHERE FABRICATION OF STRUCTURAL LOAD-BEARING MEMBERS AND ASSEMBLIES IS BEING PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTION OF THE FABRICATED ITEMS SHALL BE REQUIRED BY TABLE 2 AND AS REQUIRED ELSEWHERE IN THE SPECIAL INSPECTION PROGRAM. REFERENCE SECTION 1704.2.2 FOR APPROVED FABRICATOR EXCEPTION.
		CONCRETE			
REINFORCING STEEL	1704.4	ACI 318 1.3.2.C ACI 318 3.5 ACI 318 7.1 TO 7.7		Х	TOLERANCE AND REINFORCING PLACEMENT PER ACI 7.5
WELDING REINFORCING STEEL	1704.3.1 1903.1	ACI 318 3.5.2 AWS D1.4, SECTION 7			REFER TO STEEL FOR WELDING REQUIREMENTS
PLACEMENT OF CAST IN PLACE ANCHOR BOLTS	1704.4 1911.1 1912.1	ACI 318 1.3.2.C ACI 318 21.1.8	х		ALL BOLTS VISUALLY INSPECTED
VERIFYING USE OF REQUIRED MIX DESIGN(S)	1704.4 1904 1905.2-4 1913.2 1913.3	ACI 318 1.3.2.A ACI 318, CHAPTER 4 ACI 318 5.2-5.4		Х	
CONCRETE PLACEMENT, NON-SHRINK GROUT	1704.4 1905.9-10	ACI 318 1.3.2.D ACI 318 5.9-5.10	х	·	
CONCRETE CURING	1704.4 1905.11-13 1913.9	ACI 318 1.3.2.D ACI 318 5.11-5.13		X	
REINFORCING STEEL MECHANICAL COUPLERS, TERMINATORS AND FORMSAVERS		ICC EVALUATION REPORTS		х	
		STEEL	·		
FABRICATION OF STRUCTURAL ELEMENTS	1704.2			X	REFER TO INSPECTION OF FABRICATOR REQUIREMENTS
MATERIAL VERIFICATION OF STRUCTURAL STEEL	1704.3 2203.1	ASTM A6 AISC 360 A3.1 AISC 360 M5, 5		X	CERTIFIED MILL TEST REPORTS
MATERIAL VERIFICATION OF HIGH STRENGTH BOLTS, NUTS, AND WASHERS	1704.3.3	AISC 360 A3.3		Х	MANUFACTURER'S CERTIFIED TEST REPORTS
MATERIAL VERIFICATION OF ANCHOR BOLTS AND THREADED RODS	1704.3	AISC 360 A3.4		X	MANUFACTURER'S CERTIFIED TEST REPORTS
MATERIAL VERIFICATION OF WELD FILLER METALS	1704.3.1	AISC 360 A3.5		Х	MANUFACTURER'S CERTIFIED TEST REPORTS
VERIFYING USE OF PROPER WPS'S				Х	COPY OF WELDING PROCEDURE SPECIFICATIONS
VERIFYING WELDER QUALIFICATIONS				Х	COPY OF QUALIFICATION CARDS

		INSPECTION			
SYSTEM OR MATERIAL	IBC CODE REFERENCE	CODE OR STANDARDS REFERENCE	FREQUENCY (NOTE 5)		REMARKS
			CONTINUOUS	PERIODIC	
COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS			х		ALL WELDS VISUALLY INSPECTED PER AWS D1.1 6.9
MULTIPASS FILLET WELDS	1704.3.1 1704.4	AWS D1.1 SECTION 6	Х		
SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16"		SECTION		×	
SNUG-TIGHT HIGH STRENGTH BOLT INSTALLATION	1704.3	RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS SECTION 9 AISC 360 M2.5		x	ALL CONNECTIONS VISUALLY INSPECTED
VERIFICATION OF FRAME JOINT DETAILS INCLUDING MEMBER AND COMPONENT LOCATIONS, BRACING, AND STIFFENERS	1704.3.2			X	
MATERIAL VERIFICATION OF WELD FILLER METALS	·			Х	MANUFACTURER'S CERTIFIED TEST REPORTS
VERIFYING USE OF PROPER WPS'S				х	COPY OF WELDING PROCEDURE SPECIFICATIONS
VERIFYING WELDER QUALIFICATIONS				X	COPY OF QUALIFICATION CARDS
	POS	T INSTALLED CONCRETE A	NCHORS		
EXPANSION ANCHORS INSTALLATION IN HARDENED CONCRETE AND COMPLETED MASONRY	1703.4.2 1704.15	ICC EVALUATION REPORT	X	X (NOTE 6)	INSPECTION REQUIREMENTS PER ICC EVALUATION REPORT
EPOXY ANCHORS INSTALLATION IN HARDENED CONCRETE AND COMPLETED MASONRY	1912.1	ACI 318: 3.8.6. 21.1.8	Х		INSPECTION REQUIREMENTS PER ICC EVALUATION REPORT

TESTING

TABLE 3 - REQ	UIRED TE	ESTING FO	R SPE	CIAL I	NSPECTIONS		
		TESTING					
SYSTEM OR MATERIAL	IBC CODE REFERENCE	CODE OR STANDARD	FREQUENCY		ANDARD		REMARKS
		REFERENCE	CONTINUOUS	PERIODIC			
		GEOTECHNICAL					
TEST PILES					BY THE GEOTECHNICAL ENGINEER		
		CONCRETE					
CONCRETE STRENGTH	1903	ASTM C39	EACH 150 CY N		FABRICATE SPECIMENS AT TIME FRESH		
CONCRETE SLUMP	1704.4 1905.6	ASTM C143	SLAB OR WAI		CONCRETE IS PLACED		
CONCRETE AIR CONTENT		ASTM C231	EACH DAY				
CONCRETE TEMPERATURE		ASTM C1064					
		STEEL	·				
MAGNETIC PARTICLE (MT) AND ULTRASONIC (UT) TESTING OF WELDS		MT - AWS D1.1 6.14.4 UT - AWS D1.1 6.13 & 6.14.3	PER DRAWING	S	ALL C.J.P. WELDS REQUIRE UT TESTING		

STATEMENT OF SPECIAL INSPECTION NOTES:

- SPECIAL INSPECTIONS SHALL CONFORM TO SECTION 1704 OF THE 2010 OSSC. REFER TO TABLES 1 AND 2 FOR SPECIAL INSPECTION AND TABLE 3 FOR TESTING REQUIREMENTS.
- 2. SPECIAL INSPECTIONS AND ASSOCIATED TESTING SHALL BE PERFORMED BY AN APPROVED ACCREDITED INDEPENDENT AGENCY MEETING THE REQUIREMENTS OF ASTM E329 (MATERIALS), ASTM D3740 (SOILS), ASTM C1077 (CONCRETE), ASTM A880 (STEEL), AND ASTM E543 (NON-DESTRUCTIVE). THE INSPECTION AND TESTING AGENCY SHALL FURNISH TO THE STRUCTURAL ENGINEER A COPY OF THEIR SCOPE OF ACCREDITATION. SPECIAL INSPECTORS SHALL BE CERTIFIED BY THE BUILDING OFFICIAL. WELDING INSPECTORS SHALL BE QUALIFIED PER SECTION 6.1.4.1.1 OF AWS D1.1.
- 3. THE SPECIAL INSPECTOR SHALL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTOR FOR CORRECTION AND NOTED IN THE INSPECTION REPORTS.
- 4. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT STATING THAT THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED.
- 5. CONTINUOUS INSPECTION: THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED.

PERIODIC INSPECTION: THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK.

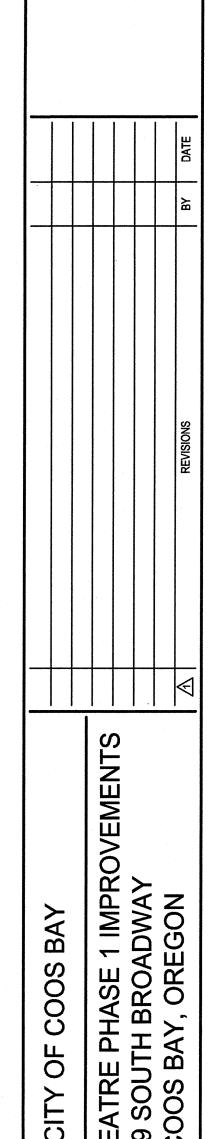
6. WHERE PERIODIC INSPECTION IS ALLOWED IN ACCORDANCE WITH AN EXPANSION ANCHOR'S ICC EVALUATION REPORT, INSPECTIONS SHALL BE AS

FOR ALL ANCHORS, PRIOR TO CONCEALMENT, VERIFY: ANCHOR TYPE, ANCHOR DIMENSIONS, ANCHOR SPACING AND EDGE DISTANCES.

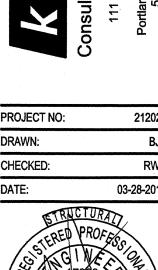
• FOR EACH ANCHOR TYPE AND SIZE, INSPECTOR SHALL BE ONSITE TO CONTINUOUSLY INSPECT A MINIMUM OF THE FIRST 10 ANCHORS INSTALLED BY EACH INSTALLER FOR CONFORMANCE WITH ICC EVALUATION REPORT. PROVIDED ALL ANCHORS ARE INSTALLED CORRECTLY PER MANUFACTURER'S INSTRUCTIONS, PROVIDE PERIODIC INSPECTION ON A MINIMUM OF 10% OF THE NEXT 1000 ANCHORS BY EACH INSTALLED AND A MINIMUM OF 5% OF THE REMAINING ANCHORS BY EACH INSTALLER. INSPECTIONS SHALL OCCUR A MINIMUM OF ONCE PER WEEK AT A RANDOM TIME WHILE ANCHOR INSTALLATION IS ONGOING. ANY NON-COMPLIANCE ISSUES SHALL RESET THE INSPECTION REQUIREMENTS TO TEN (10) CONTINUOUS INSPECTIONS. NON-COMPLIANT ANCHORS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD FOR REVIEW AND SHALL BE BROUGHT INTO COMPLIANCE BY EITHER TESTING OR RE-INSTALLATION.

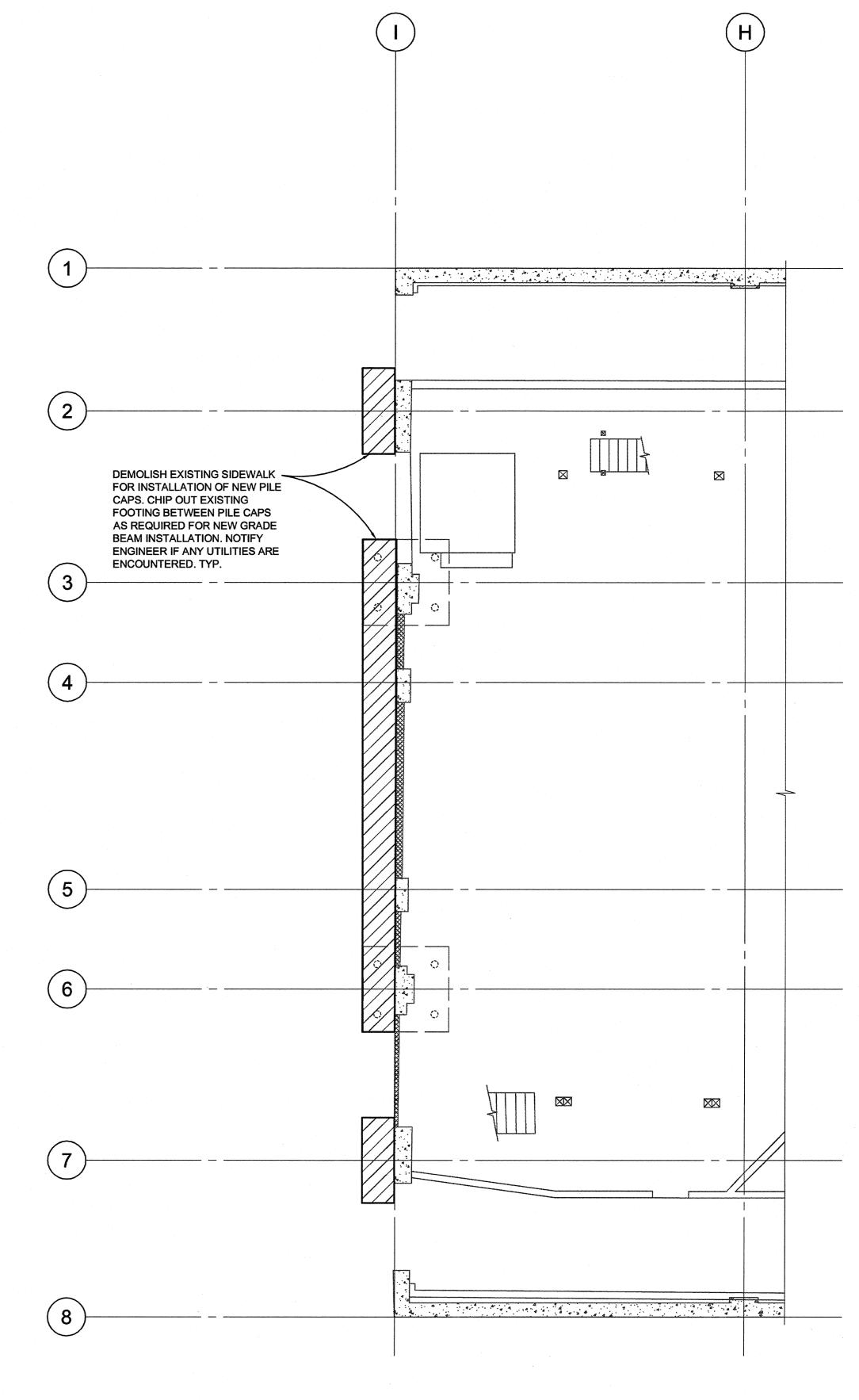
INSPECTION REPORTS SHALL IDENTIFY NAMES OF INSTALLERS.

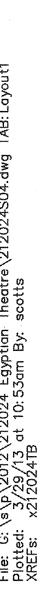
 SPECIAL INSPECTOR SHALL PROVIDE DOCUMENTATION AT THE END OF ANCHOR INSTALLATIONS STATING THAT THE MINIMUM NUMBER OF ANCHORS WERE





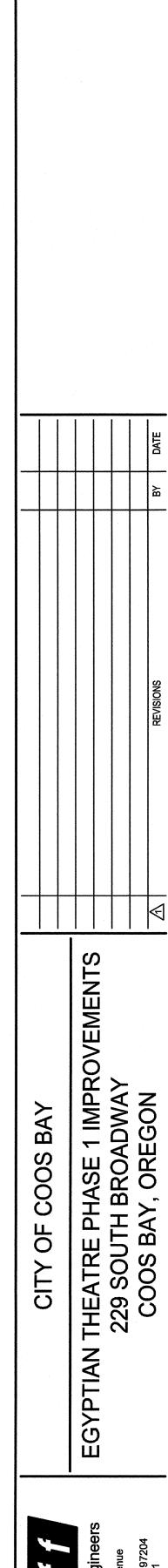






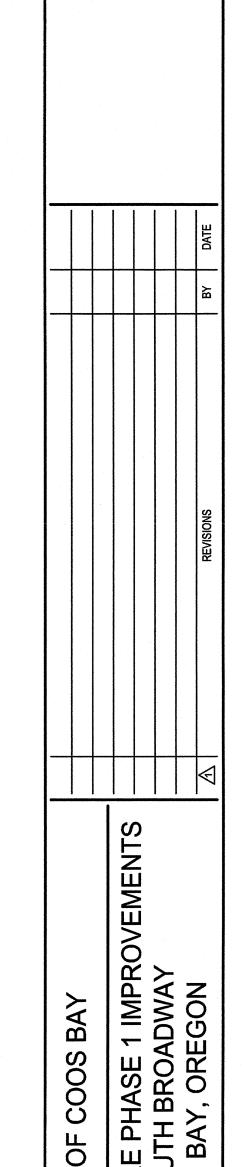
DEMOLITION PLAN

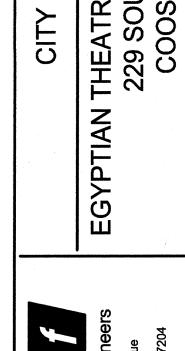
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FRAMING PLAN

1 MEZZANINE FRAMING PLAN
3/16"=1'-0"

(N) W8x35

FINISH NOTES:

WHERE NEW STEEL BEAM OCCURS BELOW EXISTING CEILING, PROVIDE GYPBOARD ENCLOSURE WITH

PATCH EXISTING CEILING AT NEW BEAM LOCATIONS TO MATCH EXISTING.

TEXTURED FINISH AND PAINT TO MATCH EXISTING

PATCH EXISTING WALLS/SOFFITS AT NEW BEAM CONNECTIONS TO EXISTING FRAMING, TYPICAL TO MATCH EXISTING.

(E) BEAM ASSUMED TO BEAR COMPLETELY ON (E)

POST VERIFY. CONSULT

(5)

INDICATES EXISTING.

____ INDICATES EXISTING STRUCTURE.

CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS,

DIMENSIONS, AND ELEVATIONS PRIOR TO FABRICATION AND ERECTION AND NOTIFY ENGINEER OF ANY

SIGNIFICANT DISCREPANCIES FROM THAT SHOWN ON

CONTRACTOR TO SHORE ALL EXISTING FRAMING AS REQUIRED FOR DEMOLITION AND REFRAMING WORK.

INDICATES NEW.

THE DRAWINGS.

6. INDICATES EXISTING CONCRETE WALL.

PLAN NOTES:

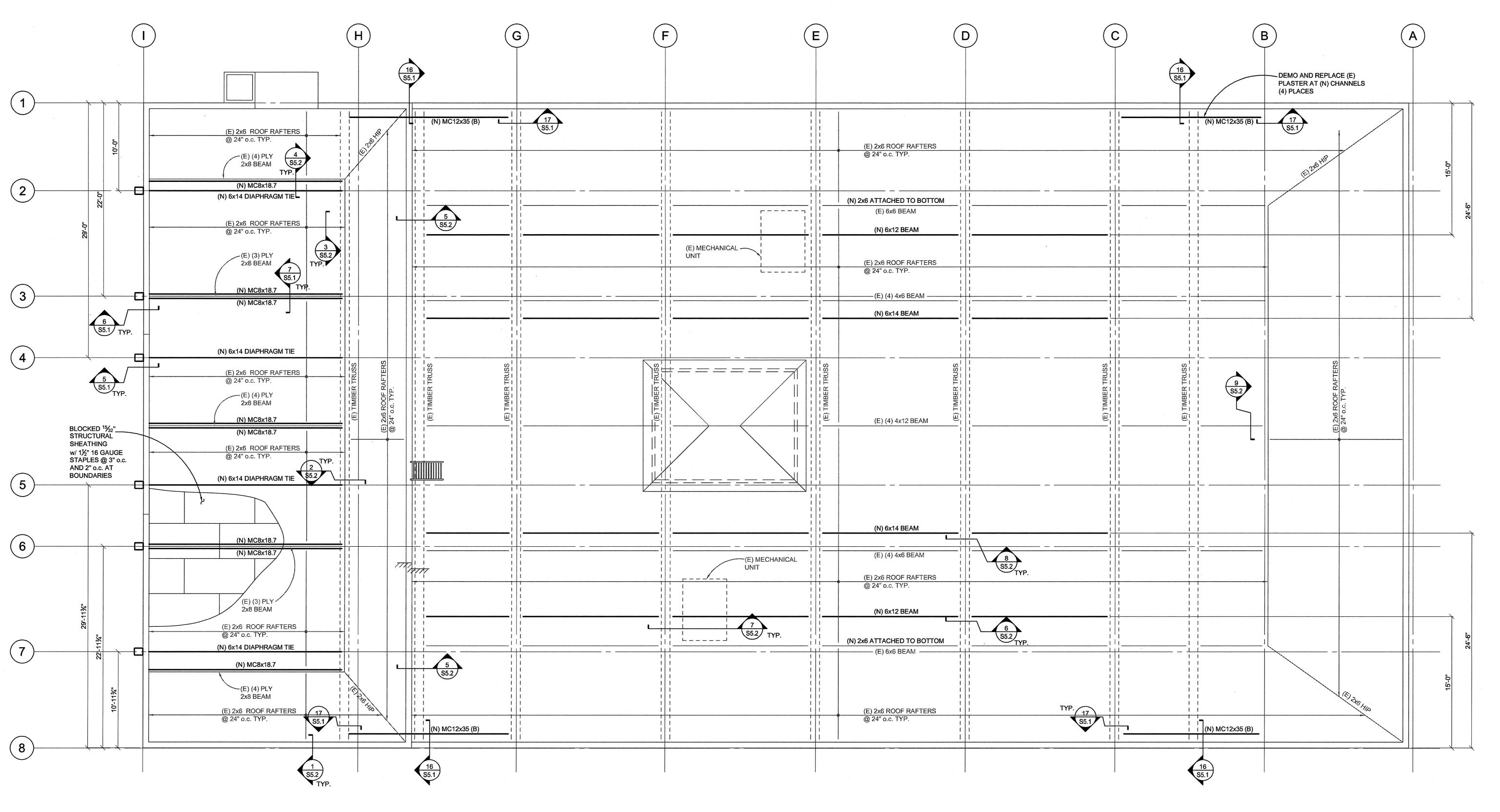
ENGINEER OF RECORD IF
BEAM SUPPORT CONDITION
VARIES FROM ASSUMED.

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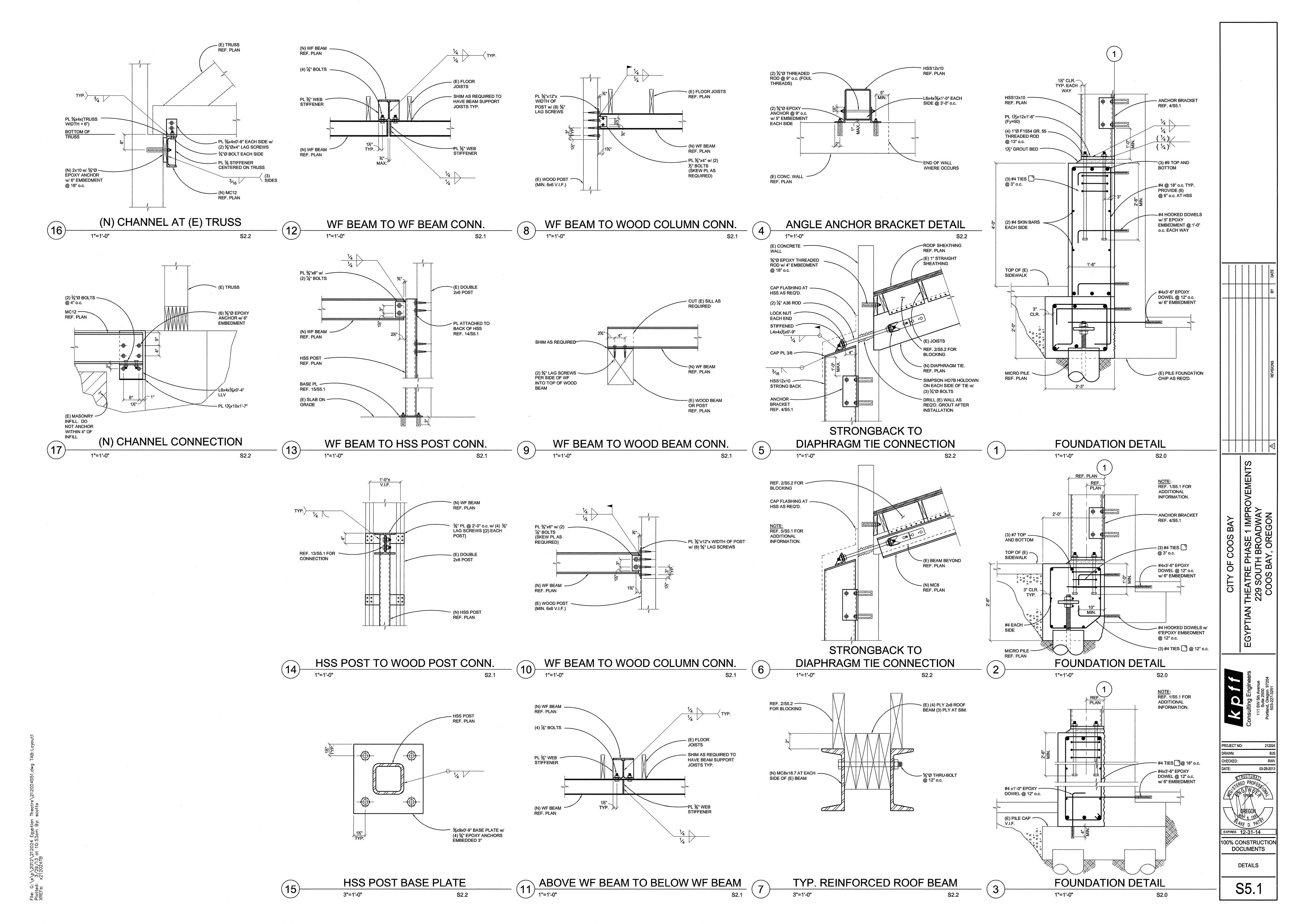
CITY OF COOS BAY

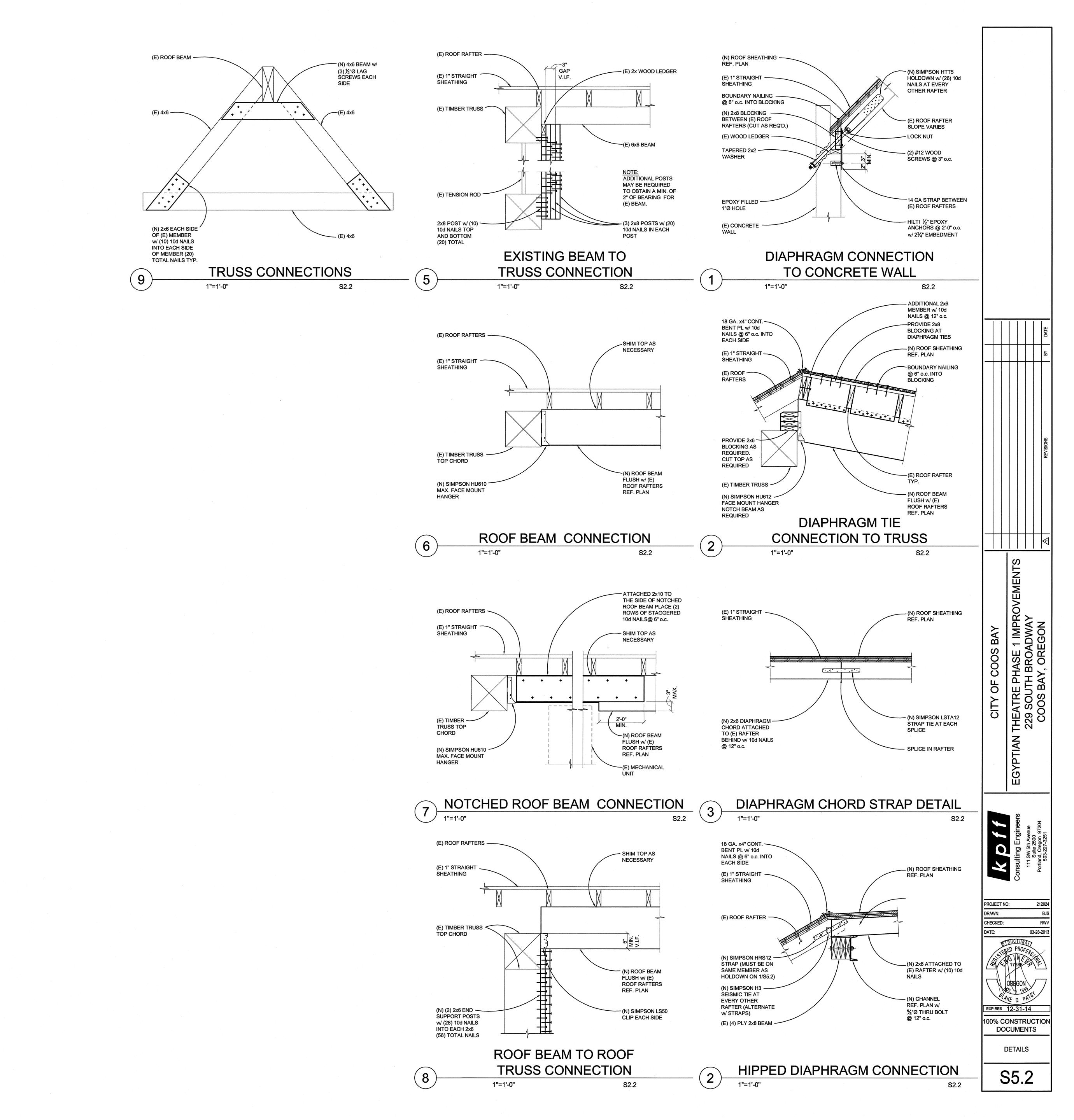
100% CONSTRUCTION DOCUMENTS

ROOF FRAMING

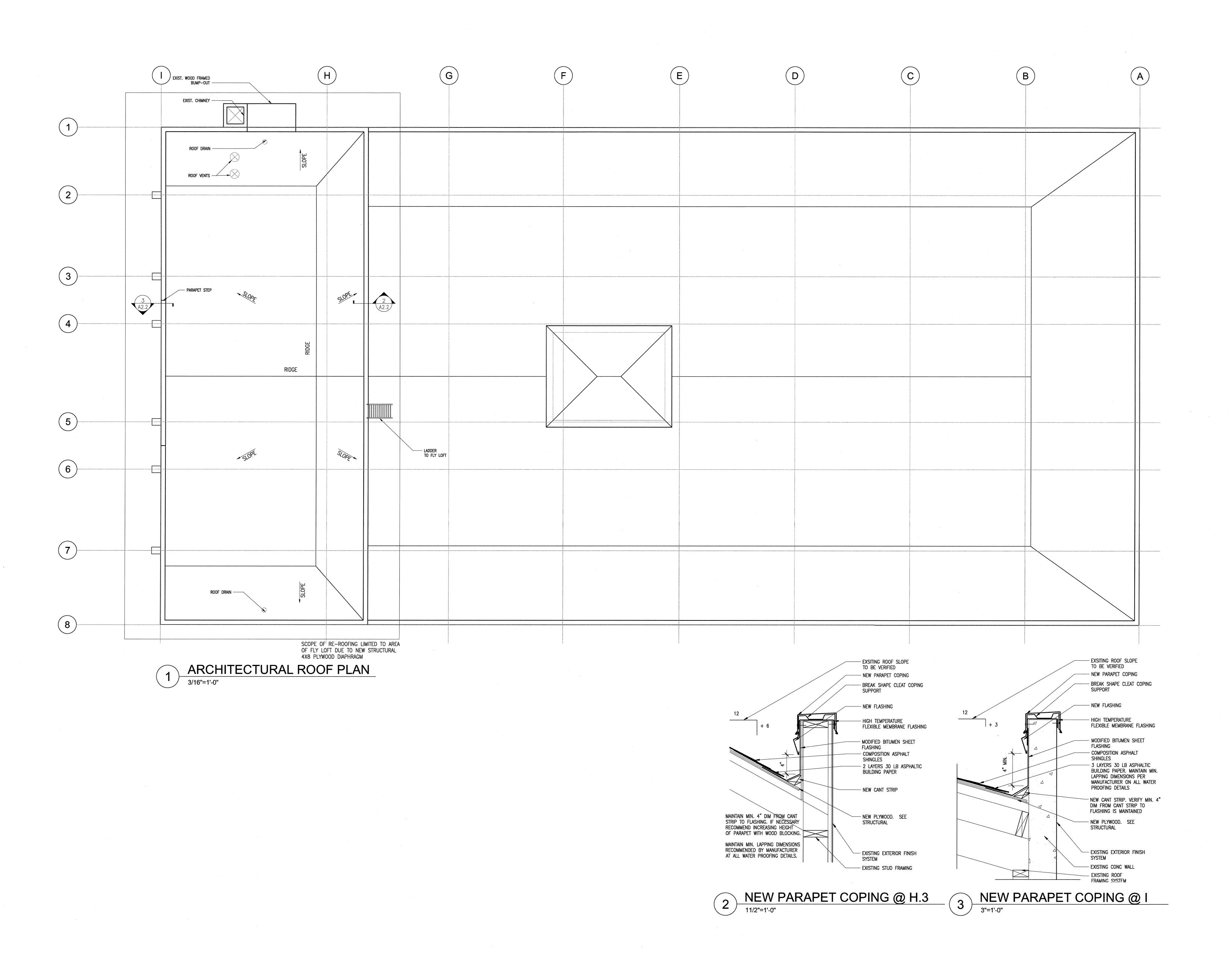


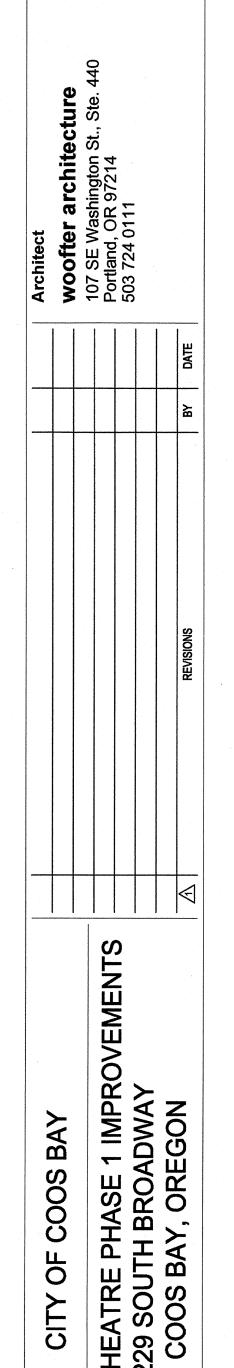
PLAN NOTES: INDICATES EXISTING. INDICATES NEW. ———— INDICATES EXISTING STRUCTURE. CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, AND ELEVATIONS PRIOR TO FABRICATION AND ERECTION AND NOTIFY ENGINEER OF ANY SIGNIFICANT DISCREPANCIES FROM THAT SHOWN ON THE DRAWINGS. CONTRACTOR TO SHORE ALL EXISTING FRAMING AS REQUIRED FOR DEMOLITION AND REFRAMING WORK. 6. INDICATES EXISTING CONCRETE WALL. INDICATES BELOW.

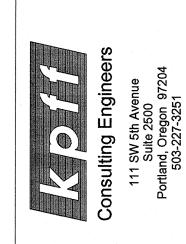


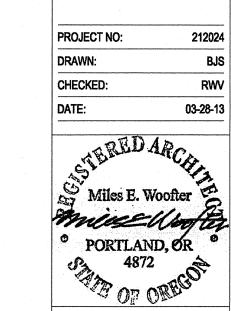


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100% CONSTRUCTION DOCUMENTS

Roof Plan

A2.2